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ANALYSIS AND DESIGN OF STEEL WAREHOUSE

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ABSTRACT:

Steel warehouse design has become increasingly recognized as a highly economical and efficient method in the realm of steel construction, particularly for low-rise and single-storey facilities. Owing to the high strength-to-weight ratio of steel compared to reinforced concrete structures, steel warehouses are widely adopted in industrial applications, allowing for larger spans and unobstructed interior spaces. This design approach focuses on optimizing structural components to achieve both structural efficiency and economic viability. When compared to conventional steel buildings (CSBs), steel warehouse systems offer several advantages, including cost-effective fabrication and faster construction timelines. In this study, the design process and load assessments for a steel warehouse are carried out in accordance with IS 875 and IS 800 standards, with additional reference to AISC-2011 and MBMA-96 guidelines, using STAAD.Pro V8i software. A hybrid design methodology is utilized to determine the most economical structural sections, drawing from established research and previous studies.

Keyword: Pre Engineered Building, AISC-2011, MBMA-96, IS 875, IS 800:2008, STAAD pro v8i

I. INTRODUCTION

The growing demand for fast, cost-effective, and durable storage solutions has led to a significant rise in the use of steel warehouse structures. These buildings offer numerous advantages such as high structural strength, reduced construction time, and long spans with minimal internal supports, making them ideal for industrial and commercial applications. Steel warehouses are designed to optimize material usage and structural performance while ensuring compliance with modern design standards and safety codes. With the aid of advanced design tools like STAAD.Pro and adherence to relevant codes such as IS 875, IS 800, AISC-2011, and MBMA-96, steel warehouse design has evolved into a highly efficient and reliable construction method. This study focuses on the structural analysis and design of a steel warehouse using a combination of design standards to achieve an economical and structurally sound solution.

II EXPERIMENTAL INVESTIGATION

A. Structure Configuration

In this study, a three-dimensional single-storey industrial building with symmetry in the X and Y directions and a rectangular shape (80m X 25m) is investigated (Table 2: Structure configuration). In the pre-engineered frame model, a steel frame with a tapered "I" section is utilized. A steel rod is used for bracing (X-type) in the models. The structure is supported by fixed support and all joints are absolutely stiff. IS: 875-1987 (Parts I, II), IS: 875-2015, IS: 800-2007 are used for load calculation.

Table 1: Structure Configuration

Location	Jagdalpur
Length	80 m
Width	25 m
Eave height	9 m
Seismic zone	Zone II
Wind speed	39 m/s
Wind terrain category	2
Wind class	C
Lifespan	50 years
Slope of roof	1 in10
Soiltype	Medium
Bay space	8 m c/c
Support condition	fixed
Importance factor	1

Response Reduction Factor	5
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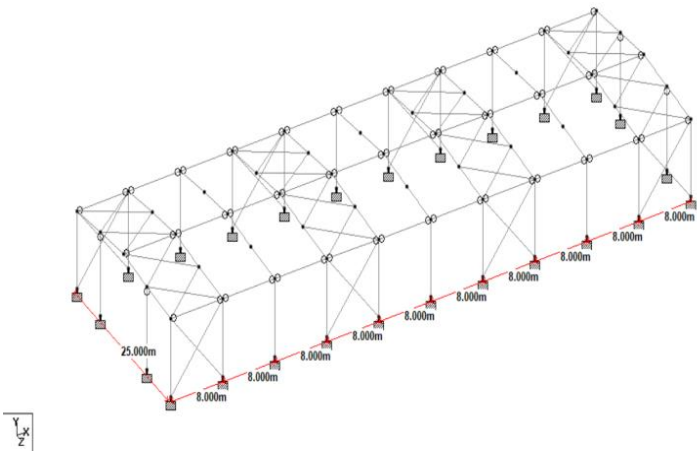


Fig.1 CSB Frame Model 3D View

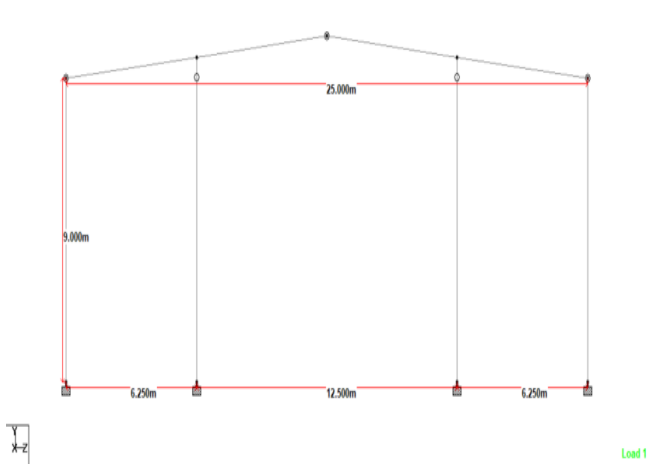


Fig.2 CSB Frame Front View

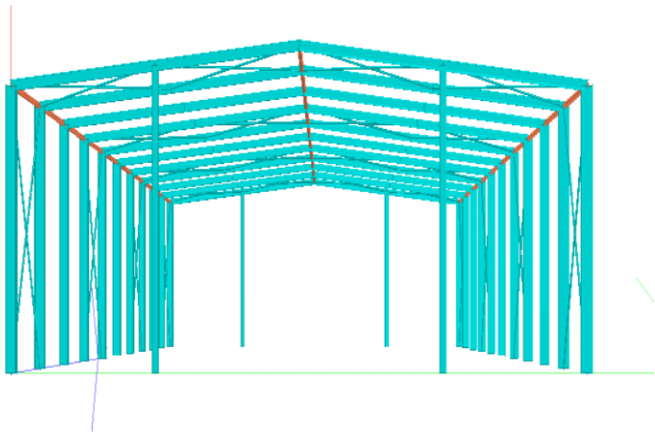


Fig.3 CSB Frame Render image



Fig.4 CSB Frame component

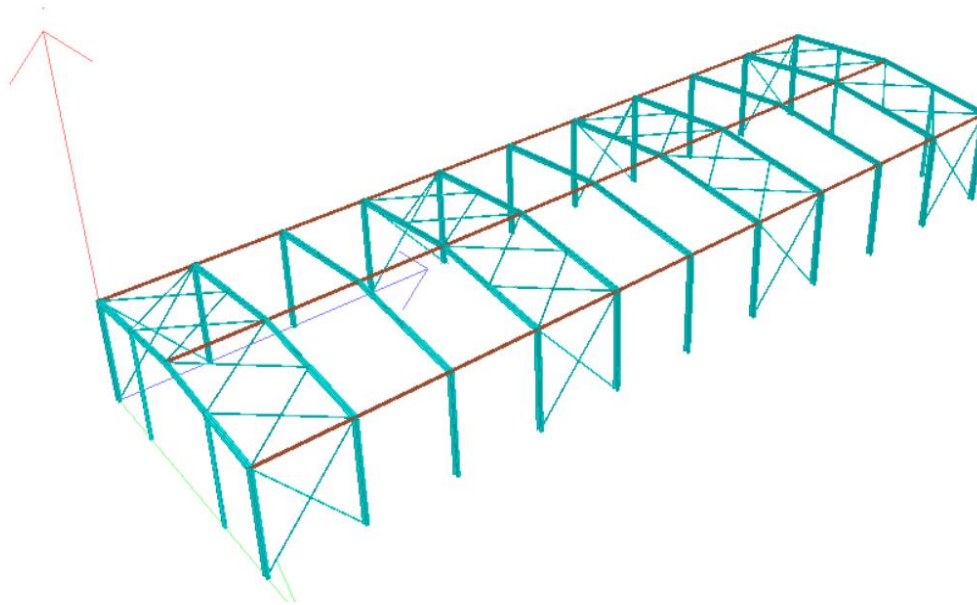


Fig.4 CSB Frame component

B. Dead Load Calculation

Sheet (Galvanized iron sheet) as per IS 875 (part 1)-1987 page 10 [13].

Thickness= 0.63 m

Sheet wt. = 0.054 KN/m²

Purlin wt. = 0.08 KN/m²

Fixtures like (sag rod, bracing etc) wt. = 0.043 KN/m²

Total Dead Load = 0.054+ 0.084+0.04 = 0.181 KN/m²

Dead load for 8 m bay = 0.181 x 8 =1.448 KN/m

For end frame = 1.448/2 = 0.724 KN/m

C. Live Load Calculation

As per IS 875 (part 2)-1987 page -14 [14]

For slope roof with slope up to 10° (including) and access not provided = 0.75KN/m²

Slope <10° no extra calculations is required

Line Load for 8m bay space = 0.75 x 8 = 6KN/m

For end frame = 6/2 =3KN/m.

D. Wind Load Calculation

As per IS 875 (part 3)-2015 [15]

K_d for triangular, square, and rectangular

$$K_d = 0.9$$

K_a value depend upon tributary area

$$K_a = 0.837$$

K_c depend upon pressure and suction of roof

$$K_c = 0.9$$

V_b , Basic wind speed (for Jagdalpur)

$$V_b = 39 \text{ m/s}$$

K_1 , Risk coefficient

For design life 50 year

$$K_1 = 1$$

K_2 , Terrain, height factor

Here terrain category=2

For height= 9 m

$$K_2 = 1$$

K_3 , Topography K_3 factor

Here slope is less than 3°

$$K_3 = 1$$

K_4 Cyclonic factor

(As jagdalpur is located > 60 km from east coast) Therefore, for industrial building with no cyclonic factor

$$K_4 = 1$$

V_z , Design wind speed at height Z

$$V_z = V_b \times K_1 \times K_2 \times K_3 \times K_4$$

$$V_z = 39 \text{ m/s}$$

P_z , Wind pressure at height Z

$$P_z = 0.913 \text{ KN/m}^2$$

P_d , Designed wind pressure

$$P_d = K_d \times K_a \times K_c$$

$$P_d = 0.64 \text{ KN/m}^2$$

“The value of P_d , however shall not be taken as less than $0.70 P_z$ ” [5].

$$F = (C_{pe} - C_{pi}) A P_d$$

where

C_{pe} = external pressure coefficient,

C_{pi} = internal pressure coefficient,

A = surface area of structural element or

cladding unit, and

P_d = design wind pressure [15].

For both pressure and suction, external and internal wind factors are determined for all surfaces. Because the opening in the building is smaller than 5%, the internal coefficients are +0.2 and -0.2, respectively[15].

E. Load Combination

Here the load combination is taken as per IS 800:2007 and not as per AISC because as location of the project the loading conditions depends and load combination by IS 800:2007 gives higher value which contribute towards safety of structure. As per AISC-2011, there is no need for different load combinations for strength and deflection[15].

DL+LL

DL+WL

DL+0.8LL+0.8WL

1.5(DL+LL)

1.5(DL+WL)

(0.9DL+1.5WL)

1.2(DL+LL) +0.6WL

1.2(DL+LL+WL)

DL denotes Dead Load, LL denotes Live Load and WL denotes Wind Loads.

IV RESULT AND DISCUSSION

In this case, the output of the process—pictures—is an attempt to depict the outcomes of the STAAD Pro v8i program.

A. Utilization ratio check

In the construction business, Steel offer quicker, more effective construction with optimal steel utilisation, utilising Indian load calculations.

B. Reaction at supports and Bending Moment about Z due to DL

Support reaction of single frame generated due to applied load and load combinations are represented below in pictorial form.

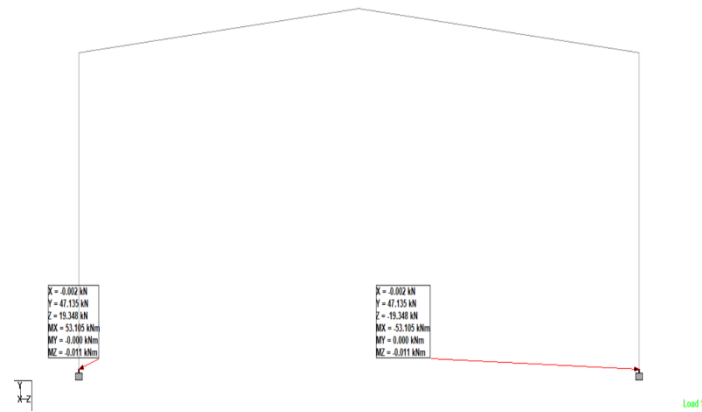


Fig.6 Support Reactions

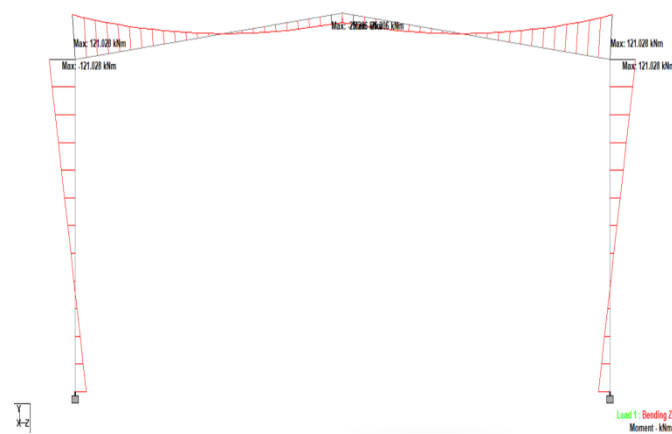


Fig.7 Bending Moment about Z axis due to DL

C. Shear about Y axis

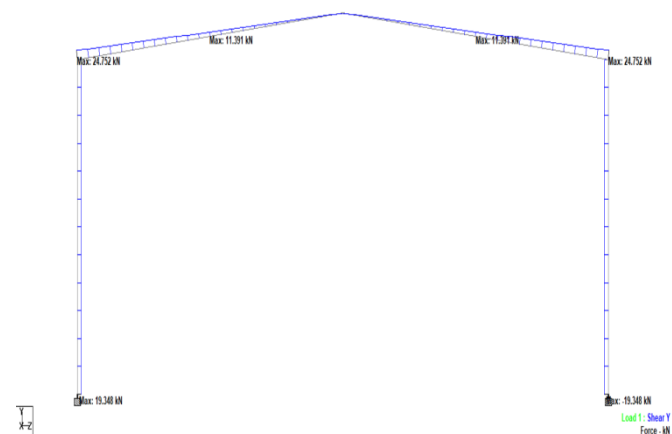


Fig.8 Shear about Y axis

D. Deflections

Deflection Limit

S.No	Description	Vertical	Horizontal
1	Main frame	L/180	H/60
2	Wind column		H/120
3	Mezzanine Beam	L/240	L/325
4	Purlin	L/180	L/180
5	Girt	L/120	L/180
6	Minimum Thickness	4 1.6	

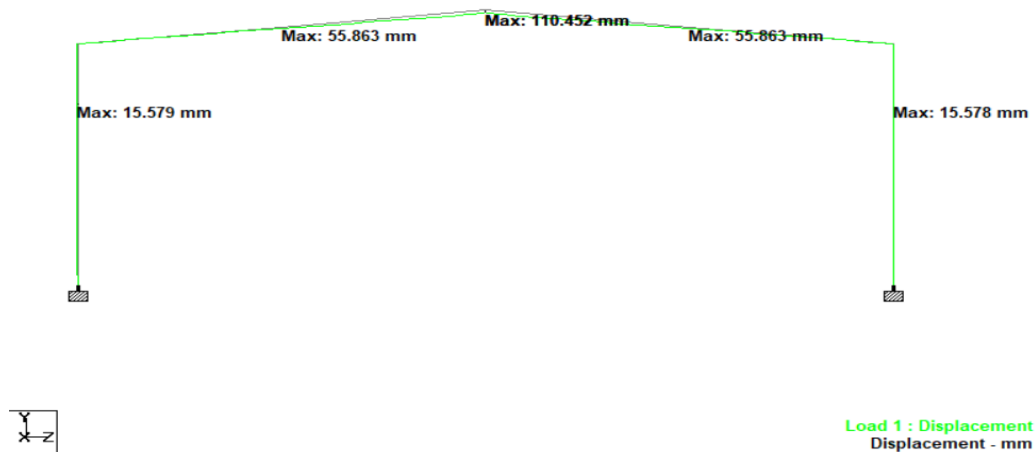


Fig.9 Displacement under LL&DL

E. Steel Take off

Steel take off is the quantity of steel to be used for the designed construction project. It generally provide the detail of the material to be consumed in the project. These features of STAAD pro enable us to estimate the quantity of material as per code standard [5].

STEEL TAKE-OFF

PROFILE		LENGTH (METE)	WEIGHT (KN)
Tapered	MembNo: 1	198.00	150.582
Tapered	MembNo: 3	138.19	104.031
Tapered	MembNo: 5	138.19	89.169
Tapered	MembNo: 64	38.50	19.520
D ISMC125		240.00	61.210
ST RD55		518.14	94.732
TOTAL =			519.243

***** END OF DATA FROM INTERNAL STORAGE *****

Fig.10 Steel take off

V. CONCLUSION

- In this type of design is adopted where IS 875:1987 part 1,part 2 ,875:2015 part 3 and IS 800:2007 is used for load calculation and load combination
- The amount of Steel take off = 519.243 KN

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